

W.S.M. → Working stress method,
Based on elastic theory.

$$\text{Stress} = \frac{\text{load}}{\text{Area}}$$

- Stresses are considered for normal working load condition.
- Stresses are limited to a fraction of stress at which material fails when tested
- F.O.S = Yield stress

Working stress

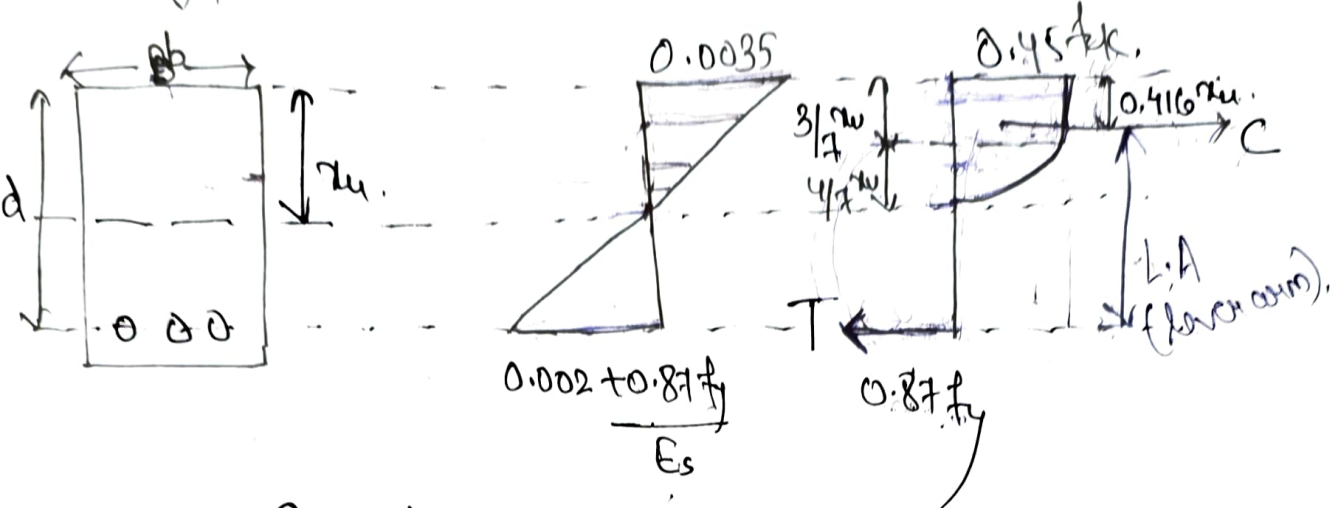
2) Load factor method or Ultimate load method

→ Ultimate load is known multiple of maximum working load.

$$\text{Load factor} = \frac{\text{Collapse load}}{\text{Working load}}$$

- Due to large reserve strength in plastic region, the resulting member is very Slender or thin, thus give rise to deformation & crack.
- Does not consider Serviceability requirements

Singly Rf Beam



Comp force = $0.36 f_{ck} x_u b$

Tensile force = $0.87 f_y A_{st}$

$$\frac{x_{u,max}}{d} = \frac{700}{1100 + 0.87 f_y}$$

For diff grade of steel -

f_y	250	415	500
$\frac{x_{u,max}}{d}$	0.53	0.48	0.46

ie $x_{u,max}$ $0.53d$ $0.48d$ $0.46d$

Singly R/F Rectangular Beams -

For equilibrium of forces -

$$\text{Comp force} = \text{Tensile force.}$$

$$\text{i.e. } 0.36 f_{ck} x_u \cdot b = 0.87 f_y \cdot A_{st}$$

$$\text{i.e. } x_u = \frac{0.87 f_y \cdot A_{st}}{0.36 f_{ck} \cdot b}$$

$$\text{OR } \frac{x_u}{d} = 2.417 \cdot p_t \cdot f_y / f_{ck}$$

$$\text{where } p_t = \frac{A_{st}}{b \cdot d} \text{ (\% reinforcement)}$$

Three cases may arise -

Case 1 x_u/d less than limiting value.

if $x_u < x_{u,lim}$, then section is underreinforced

$$\begin{aligned} \therefore \text{Moment of Resistance} &= T \cdot \text{Lever arm} \\ &= T (d - 0.42 x_u) \end{aligned}$$

$$M \cdot R = 0.87 f_y \cdot A_{st} (d - 0.42 x_u)$$

~~OR H.R.~~ i.e. stress in steel reaches to its yield value before concrete reaches its ultimate strain.
The type of failure here is ductile.

Case II If $x_u = x_{u,lim}$.

(3)

This case corresponds to Balanced section, in which the steel reaches its yield stress at the same instant when conc reaches its ultimate strain.

∴ Moment of Resistance is given by -

$$M.R_{lim} = 0.36 f_{ck} (x_{u,lim}) b \cdot (d - 0.42 x_{u,lim})$$

$$M.R_{lim} = 0.87 f_y \cdot A_{st} (d - 0.42 x_{u,lim})$$

Case III If $x_u > x_{u,lim}$

This case corresponds to over reinforced section. Max strain in conc reaches before yielding of steel. Hence conc fails first and failure is sudden. This type of failure should be avoided.

Hence an over r/f section is not allowed.

$$M.R = 0.36 \cdot f_{ck} \cdot b \cdot x_{u,lim} (d - 0.42 x_u)$$

Design of Singly R/F Beam.

(3)

1) When size of beam is not known -

To determine - b, d and A_{st} .

(* Always a limiting section should be designed.)

Step 1 - Determine limiting depth of N.A

$$\frac{x_{u,lim}}{d} = \frac{f_{sc}}{1100 + 0.87 f_y} \quad (\text{OR from Grade of steel})$$

Step 2 Select d/b ratio between 1/5 to 3.

Step 3 Find 'd' from -

$$M_{u,lim} = 0.36 f_{ck} x_{u,lim} \cdot b \cdot (d - 0.42 x_{u,lim})$$

$$\text{OR } M_{u,lim} = 0.36 f_{ck} \frac{x_{u,lim}}{d} \left(1 - 0.42 \frac{x_{u,lim}}{d}\right) b \cdot d^2$$

Step 4 Knowing value of 'b' & 'd', determine Area of reinforcement from -

$$M_{u,lim} = 0.87 f_y A_{st} (d - 0.42 x_{u,lim})$$

Analysis of T-Beam

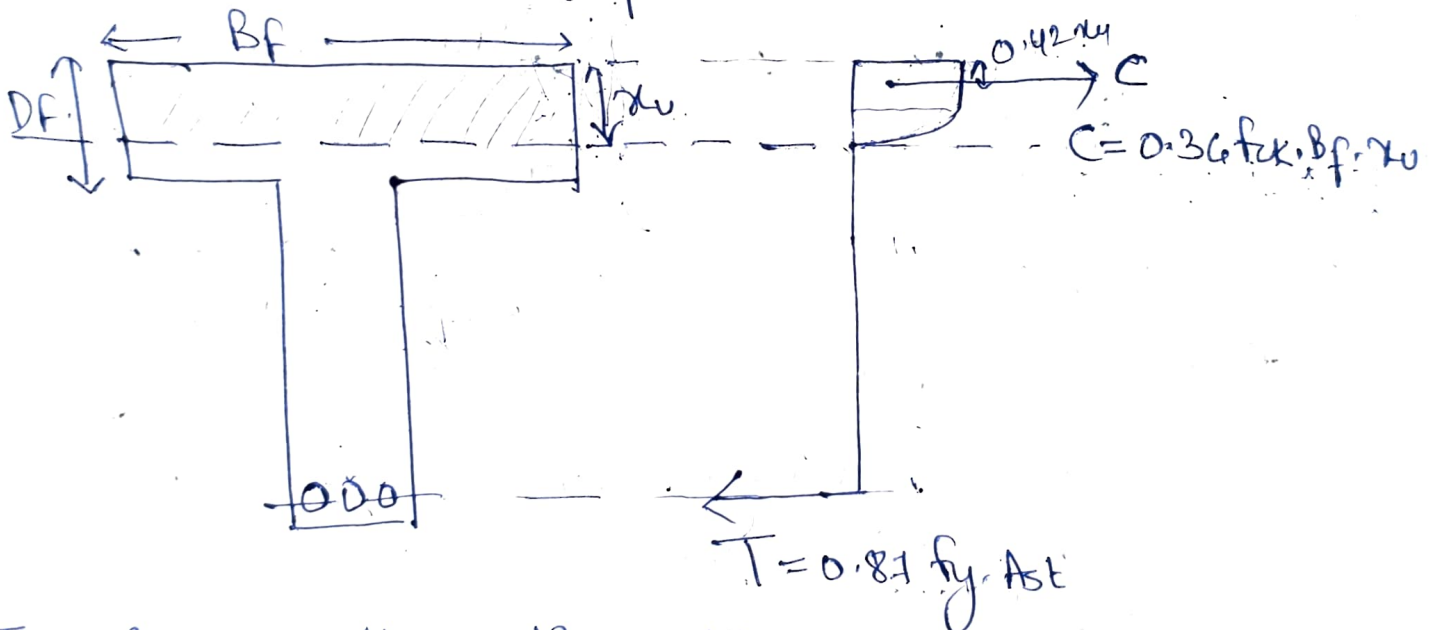
33

1) Limiting Depth of Neutral Axis -

$$\begin{aligned}x_{u,lim} &= 0.53d \rightarrow f_c - 250 \\ &= 0.48d \rightarrow f_c - 415 \\ &= 0.46d \rightarrow f_c - 500\end{aligned}$$

2) Actual Depth of N.A and Moment of Resistance -

Case (P) When N.A lie in flange Area -
ie $x_u < D_f$



In this case, the section will behave as rectangular section with width = B_f .

∴ 1) Total comp force, $C = 0.36 f_{ck} B_f x_u$.

2) Total Tensile force, $T = 0.87 f_y A_{st}$

3) Lever arm = $(d - 0.42x_u)$

4) $x_u = \frac{0.87 f_y A_{st}}{0.36 f_{ck} B_f}$, $M.R = 0.36 f_{ck} B_f x_u (d - 0.42x_u)$
 $or = 0.87 f_y A_{st} (d - 0.42x_u)$

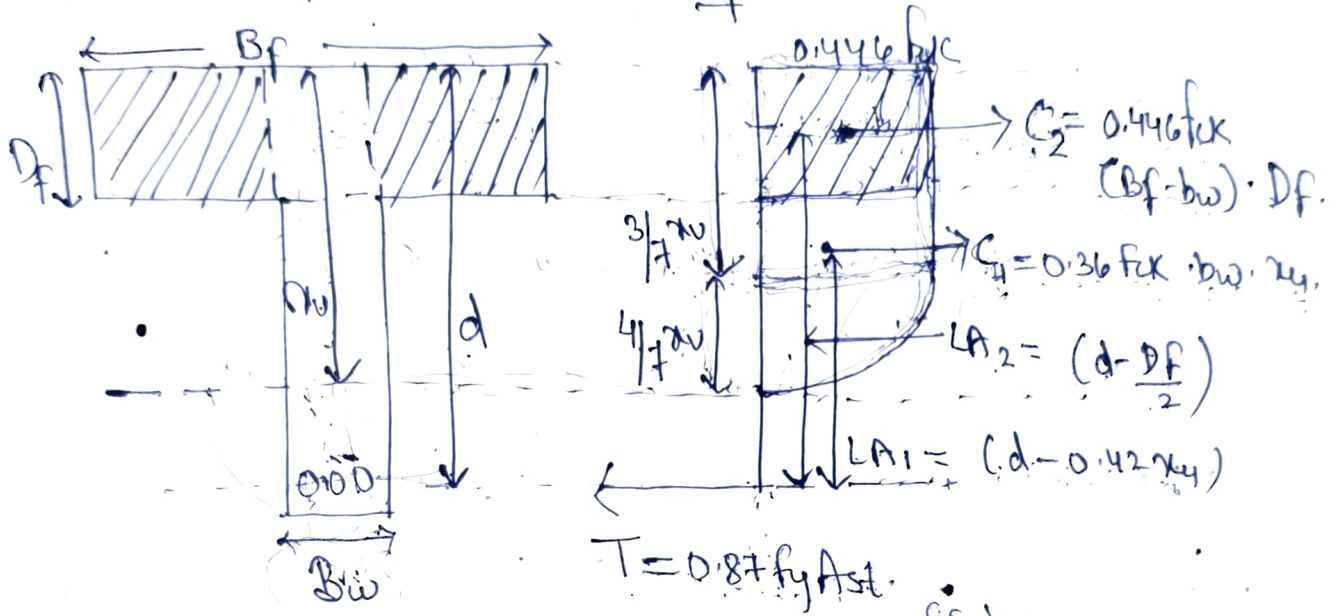
(Case ii) When N.A Res in web Area

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ie $z_u > D_f$.

Now two sub case arises -

a) $z_u > D_f$ but $d_f < \frac{3}{7} z_u$.



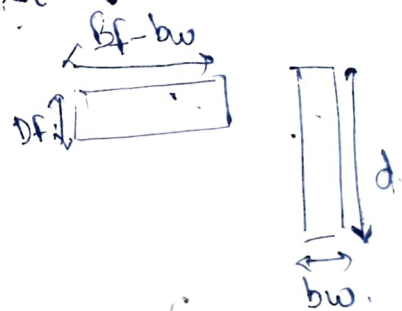
i) Total comp force

(i) for web portion -

$$C_1 = 0.36 f_{ck} \cdot b_w \cdot z_u$$

$$LA_1 = (d - 0.42 z_u)$$

$$T_1 = 0.87 f_y A_{st1}$$



ii) for flange portion -

$$C_2 = 0.446 f_{ck} (B_f - b_w) \cdot D_f$$

$$LA_2 = (d - \frac{D_f}{2})$$

$$T_2 = 0.87 f_y A_{st2}$$